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Environmental Sciences

Landscape Architecture

Land Planning

CONCEPTUAL SEWER REPORT FOR THE PLATEAU DEVELOPMENT

Carson City, Nevada

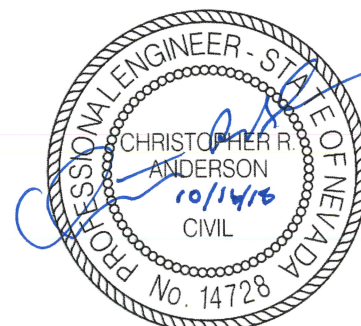
Prepared for:

TAHOE IV LLC (MR. KEITH SERPA)
P.O. BOX 1724
CARSON CITY, NEVADA 89702

Prepared by:

Manhard Consulting, Ltd.
241 Ridge Street, Suite 400, Reno, NV 89501

Job No. TIVCCNV01
October 16, 2018



Exp:12/31/18



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ABBREVIATIONS

ac	Acre
ac-ft	Acre-feet
bgs	Below ground surface
CCMC	Carson City Municipal Code 2005
cfs	Cubic feet per second
d/D	depth to diameter ratio
EDU	Equivalent dwelling unit
FEMA	Federal Emergency Management Agency
FIRM	Flood Insurance Rate Map
ft	Foot
ft ²	Square foot
ft ³	Cubic foot
fps	Feet per second
GIS	Geographical Information System
gpm	Gallons per minute
gpd	Gallons per day
Max.	Maximum
Min.	Minimum
NV	Nevada
Q _p	Peak flow
RCBC	Reinforced Concrete Box Culvert
ROW	Right of Way
vel.	Velocity



1 INTRODUCTION

1.1 PURPOSE OF STUDY

This report presents the data, methodology, and results of a conceptual sewer design report for the Plateau Development tentative map and zone change to remainder parcels. Adjacent remainder parcels that are being rezoned to multi-family and General Commercial are included in the analysis.

Future Final Maps and final designs will incorporate detailed sewer modeling and design.

This report documents the existing and proposed sewerage conditions of the proposed project:

- Existing and proposed sewershed
- Existing and proposed sewer generation rates
- Existing and proposed zoning
- Compliance with Carson City Municipal Code 2005 (CCMC) and/or other applicable ordinances

1.2 PROJECT LOCATION AND DESCRIPTION

The Plateau Development project site is in eastern Carson City, south of U.S. Highway 50 around Drako Way, located in Township 15 North, Range 20 East in portions of Sections 1 and 12.

Sanitary sewer infrastructure does not currently exist at the subject site. The nearest sanitary sewer is an 8-inch sewer main at the east end of the Morgan Mill Road that connects to the Morgan Mill sewer lift station.

Figure 1-1 shows the location of the project site. The site is not located in a FEMA flood zone. Relevant FEMA flood maps define the area as *outside the 0.2% annual chance flood*. Exhibit 1 shows the FEMA flood zone mapping adjacent to the Plateau Development.



Figure 1-1: Plateau Development Location Map



[Google Maps: <https://www.google.com/maps/search/google+maps/@39.1897644,-119.7016629,4292m/data=!3m1!1e>: accessed 10/10/2018]

2 METHODOLOGY AND ASSUMPTIONS

The existing zoning at the Plateau Development site is General Industrial. The proposed zone change and tentative map include the following land uses that constitute the sewershed for the Plateau Development and the remainder parcels:

- 269 Single Family Residential lots on 67.89 acres
- 18.53 acres Multi-Family
 - Estimated 250 EDUs
- 13.81 acres General Commercial

Existing and proposed conditions sewage loading is estimated based on the 2017 *Sewer System Sewer Master Plan Update* (ATKINS, 2017). Loading estimates are calculated using the unit rates in Table 2-1.



This conceptual sewer study for the Plateau Development adheres to the Carson City Municipal Code 2005 (CCMC) requirements for conceptual sanitary sewer preliminary design 12.06.360.

Table 2-1: Sewer System Master Plan Update Wastewater Generation Rates

<i>Land Use Category</i>	<i>Average Dry Weather Flow</i>	<i>Units</i>
<i>Single-Family Residential</i>	148	(gpd/DU)
<i>Multi-Family Residential</i>	123	(gpd/DU)
<i>Office</i>	269	(gpd/acre)
<i>Commercial</i>	443	(gpd/acre)
<i>Hotel/Resort/Casino Hotels/Motels</i>	62	(gpd/room)
<i>Industrial</i>	189	(gpd/acre)
<i>Institutional</i>	1,127	(gpd/acre)
<i>School</i>	158	(gpd/acre)
<i>Prison</i>	366	(gpd/acre)
<i>Hospital</i>	2,333	(gpd/acre)

Sewage generation estimates do not include wet weather flows. This assumption is based on construction of new infrastructure, segregation of the sewer system from stormwater infrastructure, and groundwater at 100 feet bgs.

A peaking factor of 1.5 was used for average daily to average peak hourly flow (ATKINS, 2017).

2.1 PIPE SIZING CRITERIA

Pipe sizing will conform to the Carson City Municipal Code Division 15.3.2 for sewer design criteria.

- Sewer capacity when peak flow is at $d/D = 0.75$
- Min. diameter for sewer mains is 8-inches
- Min. design velocity is 2 fps
- Max. design velocity is 10 fps

Minimum slope for sewer main pipe shall be per Table 2-2.



Table 2-2: Minimum Sewer Main Pipe Slope

<i>Diameter (in)</i>	<i>Minimum Slope</i>
8	0.4% ¹
10	0.25%
12	0.19%
15	0.14%

¹Minimum slope for 8" PVC SDR-35 flexible pipe

3 CONCEPTUAL SEWER DESIGN RESULTS

This section discusses the results of the proposed conditions sewage generation, onsite and offsite pipe capacity, and the Morgan Mill lift station capacity.

3.1 SEWAGE GENERATION

The estimates for the existing zoning and proposed Plateau Development flows are contained in Appendix A. The methodology and assumptions are included in the calculated estimates. Table 3-1 summarizes the existing zoning and proposed conditions sewage generation.

Table 3-1: Existing Zoning Sewage Generation

<i>Land Use Category</i>	<i>Sewage Loading (gpd)</i>	
	<i>Ave. Day</i>	<i>Peak Hourly</i>
<i>General Industrial</i>	21,298	31,948

Table 3-2 summarizes the proposed sewage generation values.

Table 3-2: Proposed Plateau Development and Zone Change Sewage Generation

<i>Land Use Category</i>	<i>Sewage Loading (gpd)</i>	
	<i>Ave. Day</i>	<i>Peak Hourly</i>
<i>Single Family (SF6)</i>	39,812	59,718
<i>Multi-Family</i>	30,750	46,125
<i>General Commercial</i>	6,029	9,044
<i>Park/Open Space</i>	0	0
Total	76,591	114,887

The proposed conditions will increase the average day sewage loading by 55,333 gpd.



3.2 PLATEAU DEVELOPMENT PIPE CAPACITY

Pipe capacity has been estimated for the single family residential for average daily flow and peak hourly flow. The results show that the average day and peak hourly flow are contained below the maximum 0.75 d/D ratio for 8-inch PVC pipe at the minimum slope of 0.4%. Flow capacity estimate results for 8-inch PVC pipe at 0.4% slope are summarized as follows:

- d/D=16.9% for SFR ADF=27.6 gpm
- d/D=20.6% for SFR PHF=41.5 gpm

Design velocity is less than 2 fps for both ADF and PHF at 1.6 fps and 1.8 fps, respectively. This indicates that pipe will need to be designed at slopes greater than the minimum specified in the CCMC.

The manning's normal depth calculations are included in Appendix B.

The 2017 *Sewer System Sewer Master Plan Update* includes a full buildout analysis for sewer mains and assesses the capacity for potential future deficiencies (ATKINS, 2017). This buildout condition pipeline deficiency reports the sewer mains, between the proposed Plateau Development and the Morgan Mill Lift Station, as less than d/D 0.50. This implies that the loading from the Plateau Development and associated remainder parcel zone changes would not pose a sewer main capacity issue.

3.3 MORGAN MILL LIFT STATION

The sewer loading from the Plateau Development and related zone change remainder parcels flow to the Morgan Mill Lift Station. The information presented in this section was taken from the 2017 *Sewer System Sewer Master Plan Update* (ATKINS, 2017).

The *Sewer System Sewer Master Plan Update* reports the following for the Morgan Mill Lift Station.

Table 3-3: Morgan Mill Lift Station Capacity

Lift Station	Pump Capacity (gpm)	Dry Weather (gpm)	
		Existing Q_p	Buildout Q_p
Morgan Mill	450	483	897

Carson City staff and the *Sewer System Sewer Master Plan Update* report that the lift station pump is currently undersized for the existing flows. This indicates that the pumps will have to be upgraded with, or without, additional loading from the Plateau Development.



The force main is also experiencing low velocity per the *Sewer System Sewer Master Plan Update*. The lift station wet well has sufficient volume for existing flows and reportedly has issues with the sewage going septic. Additional flows from the proposed Plateau Development would help increase the cycle times at the lift station.

Table 3-4: Lift Station Emergency Storage Capacity

<i>Lift Station</i>	<i>Dry Weather</i>				
	<i>Emergency Storage (gal)</i>	<i>Existing Q_p (gpm)</i>	<i>Existing Storage (hrs)</i>	<i>Buildout Q_p (gpm)</i>	<i>Existing Storage (hrs)</i>
<i>Morgan Mill</i>	9,800	483	0.34	897	0.18

Assuming that the 80 gpm peak hour sewage loading from the Plateau Development coincides with the master plan existing peak flow, then the emergency storage time decreases to 0.29 hours.



4 CONCLUSIONS AND RECOMMENDATIONS

4.1 GENERAL CONSIDERATIONS

This study is intended to be a preliminary sewer analysis in support of the Plateau Development tentative map and remainder parcel zone changes. Further progress towards a final design of the Plateau Development site will include a master technical sewer report specific to the final site design.

This preliminary sewer design report shows that onsite and offsite sewer mains will have capacity.

The Morgan Mill Lift Station pumping capacity is less than the existing conditions peak flow. Loading from the Plateau Development would increase this discrepancy. However, the 2017 *Sewer System Sewer Master Plan Update* identifies that the lift station pumping capacity does not meet the existing peak flow.

4.2 REGULATIONS AND MASTER PLANS

The proposed improvements and the analyses presented herein are in accordance with Carson City Municipal Code 2005.

4.3 IMPACTS TO ADJACENT PROPERTIES

There are no impacts to adjacent properties regarding sanitary sewer.

4.4 STANDARDS OF PRACTICE

This study was prepared using the degree of care and skill ordinarily exercised, under similar circumstances, by reputable professional engineers practicing in this and similar localities.



5 REFERENCES

Carson City Municipal Code. (2005).
https://library.municode.com/nv/carson_city/codes/code_of_ordinances?nodeId=CA_NEMUCO2005

ATKINS. (2017). Sewer Master Plan Update Final Report. Job No. 100052963.
Reno, NV: Brian Janes, P.E.



Exhibit 1: FEMA FIRM

in administering the National Flood Insurance Program, identify all areas subject to flooding, particularly from local small size. The community map repository should be updated or additional flood hazard information.

Relevant information in areas where **Base Flood Elevations** have been determined, users are encouraged to consult the Floodway Data and/or Summary of Saltwater Elevations in the Flood Insurance Study (FIS) report that accompanies the Flood Insurance Study. The FIS report also represents the latest FEMA data shown on the map for the community. The FIS report also contains information on the flood insurance rating and should not be used as the sole source of information. Accordingly, flood elevation data presented in the FIS should be utilized in conjunction with the FIS for purposes of floodplain management.

Flood Elevations shown on this map apply only landward from the Vertical Datum of 1988 (NAVD 88). Users of this map are that coastal flood elevations are also provided in the Flood Elevations table in the Flood Insurance Study report. Flood Elevations shown in the Summary of Stillwater Elevations for construction and/or floodplain management purposes than the elevations shown on this FIRM.

Floodways were computed at cross sections and interpolated between sections. The floodways were based on hydraulic considerations and the National Flood Insurance Program. Floodway dependent floodway data are provided in the Flood Insurance Jurisdiction.

Study report for information on flood control structures

Vertical datum information for this map are referenced to the North American Vertical Datum of 1988. For information on how to compare vertical datum, visit the National Geodetic Survey website at <http://www.ngs.noaa.gov/> or contact the National Geodetic Survey at 1915 Thompson Avenue, Alexandria, VA 22304-6145. Phone: 703/293-7200. Fax: 703/293-7201. E-mail: info@noaa.gov.

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ation, description, and/or location information for **benchmark** please contact the Information Services Branch of the Survey at (301) 713-3242, or visit its website at www.firmview.gov.

detailed and up-to-date stream channel configurations for the previous FIRM for this jurisdiction. The floodplain studies and Floodway Data tables in the Flood Insurance Study for this jurisdiction may have been transferred from the previous FIRM may have been transferred to these new stream channel configurations. As a result, the floodplain studies and Floodway Data tables in the Flood Insurance Study for this jurisdiction may reflect stream channel configurations that are not current. The floodplain studies and Floodway Data tables in the Flood Insurance Study for this jurisdiction may reflect stream channel configurations that are not current.

own on this map are based on the best data available. Because changes due to annexations or de-annexations after this map was published, map users should contact officials to verify current corporate limit locations.

Map Service Center at 1-800-358-9616 for information on associated with this FIRM. Available products may include editions of Map Change, a Flood Insurance Study report,

For more information about this map or questions concerning the National Flood Insurance Program in general, please call 1-877-FEMA MAP (1-877-335-2627) or visit our website at <http://www.msc.fema.gov/>.

osite at <http://www.sma.gov/>.

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MAP NUMBER
 3200010110
 MAP REVISION
 JANUARY 16, 2001

While administering the National Flood Insurance Program, FEMA identifies all areas subject to flooding, particularly from local small size. The community map information should be updated or additional flood hazard information.

Reliable information in areas where Base Flood Elevations have been determined, users are encouraged to consult the Floodway Data and/or Summary of Stillwater Elevations in the Flood Insurance Study (FIS) report that accompanies the Flood Insurance Study. The FIS report contains information that would be aware of the FIS. The FIS report contains information that would be aware of the FIS. The FIS report contains information that would be aware of the FIS.

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Flood Elevations shown on this map apply only landward from Vertical Datum of 1988 (NAVD 88). Users of this map are aware that coastal flood elevations are also provided in the Flood Elevations table in the Flood Insurance Study report. Flood Elevations shown in the Summary of Stillwater Elevations are for construction and/or floodplain management purposes and are higher than the elevations shown on this FIRF.

roadways were computed at cross sections and interpolated between them. The floodways were based on hydraulic considerations of the National Flood Insurance Program. Floodway dependent floodway data are provided in the Flood Insurance Jurisdiction.

in the preparation of this map was Universal Transverse Mercator datum, NAD83, GRS1980 datum, sphere, projection or UTM zones used in the map. Adjacent jurisdictions may result in slight differences across jurisdiction boundaries. These differences may be due to the use of different datum, projection or UTM zones used in the map.

This map are referenced to the North American Vertical Datum of 1988. For information on how these flood elevations must be compared to structure and referenced to the same vertical datum, visit the National Geodetic Vertical Datum of 1988, visit the National Geodetic Survey or contact the National Geodetic Survey at www.ngs.noaa.gov/.

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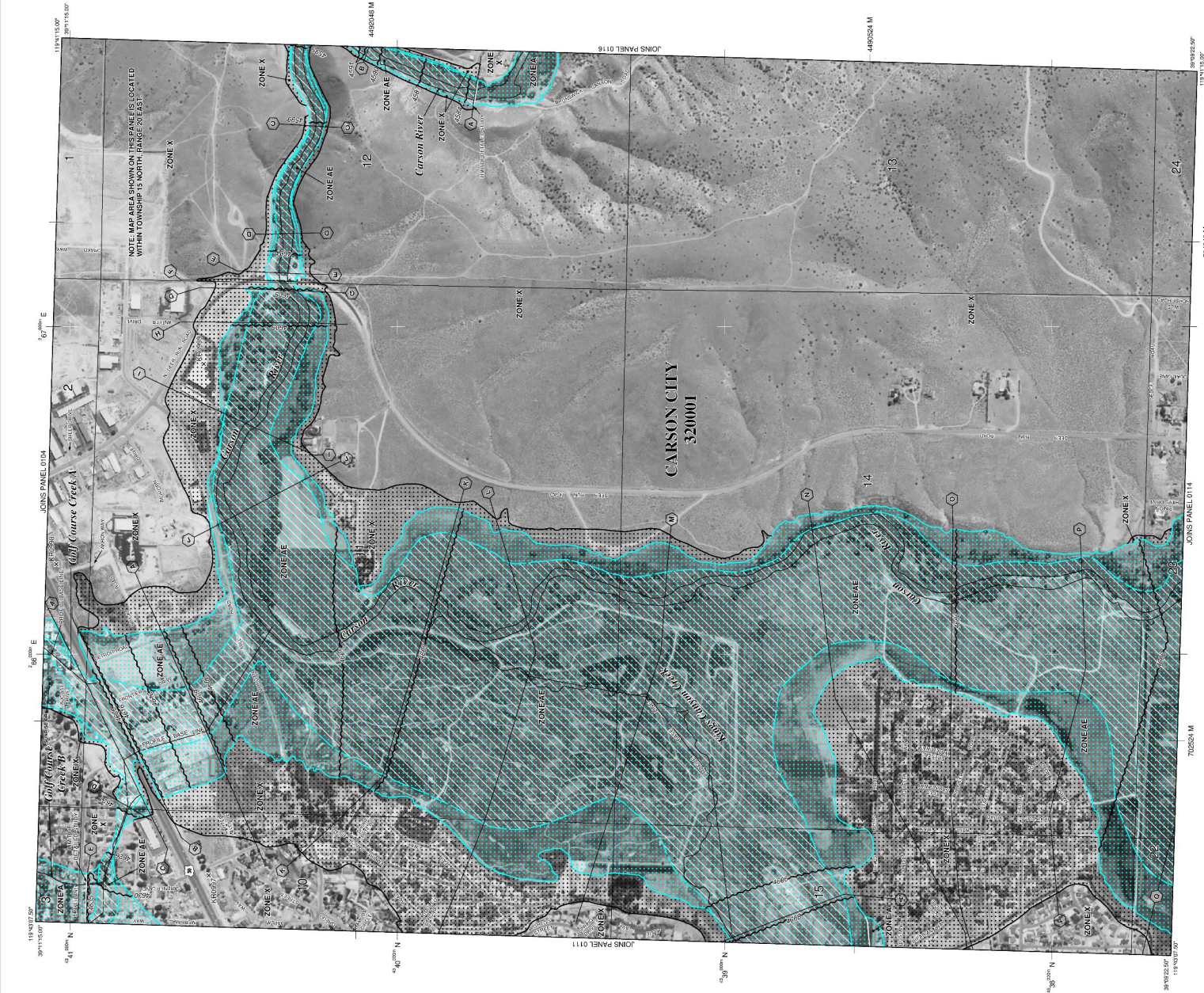
detailed and up-to-date stream channel configurations for the previous FIRM for this jurisdiction. The floodplains were transferred from the previous FIRM may have been to these new stream channel configurations. As a result, Floodway Data tables in the *Flood Insurance Act* and Floodway Data tables in the *Flood Insurance Act* may reflect stream hydraulic data may reflect stream hydraulic data.

own on this map are based on the best data available. Because changes due to annexations or de-annexations after this map was published, map users should contact local officials to verify current corporate limit locations.

Map Service Center at 1-800-358-9816 for information on products associated with this FIRM. Available products may include editions of Map Change, a Flood Insurance Study report,

ons about this map or questions concerning the National
 in general, please call 1-877-FEMA MAP (1-877-338-2627)
 be at <http://www.fema.gov>.

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Special Flood Hazard Area formerly protected from the 1% annual chance flood by a flood control system that was subsequently dismantled. Zone AE indicates that the former flood control system is being reauthorized to provide protection from the 1% annual chance or greater flood.	Zone A99 indicates that the area is to be protected from 1% annual chance flood by a Federal flood protection system under construction; no Base Flood Elevations determined.
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LINE V	Coastal flood zone with velocity hazard (wave action); no flume Flood Elevations determined.
LINE VE	Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined.
	FLOODWAY AREAS IN ZONE AE
	A Floodway is the channel of a stream plus any adjacent floodplain areas that must be

OTHER FLOOD AREAS

Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood.

OTHER AREAS
Areas determined to be outside the 0.2% annual chance floodplain.
Areas in which flood hazards are undetermined, but possible.

OTHERWISE PROTECTED AREAS (OPAs)

OPAs are normally located within or adjacent to Special Flood Hazard Areas.

- 1% annual chance floodplain boundary
- 0.2% annual chance floodplain boundary
- Floodway boundary
- Zone D boundary

The diagram illustrates a vertical cross-section of a river channel and its surrounding land. On the left, a series of dots represents the 'CBRS and OPA boundary'. To the right of this boundary, a horizontal line indicates the 'Special Flood Hazard Areas of different flooding'.

Below the flood hazard area, a wavy line represents the 'Base Flood Elevation, flood depths or flood velocities.' This line is labeled with the value '513' and '(EL 087)'.

To the right of the wavy line, a vertical axis is shown with labels: 'Base Flood Elevation the value; elevation in feet*', 'Base Flood Elevation value where uniform within zone;', and 'elevation in feet*'. The top of this axis is aligned with the 'Special Flood Hazard Areas of different flooding' line.

A note at the bottom right states: 'Based on the North American Vertical Datum of 1988 (NAVD 88)'. The entire figure is captioned as 'FIGURE 6. Relationship between CBRS and OPA boundaries.'.

97°40' 30" W
 23°55' 00" N
 Cross section line
 Transsect line
 Geographic coordinates referenced to the North American Datum of 1983 (NAD 83)
 1000-meter Transverse Mercator grid cells, zone 11

60000000 M
5000-foot grid box: Nevada
system west zone (PZONE 2003),
Size: Plane coordinate
Transverse Mercator
Bench mark (see
this FIRM panel)
DX5510
River Mile
M1.5

MAP REPOSITORY
Carson City Planning Dept., 2621 Fortgate Lane, Suite 62, Carson City, Nevada 89706 (Maps available for reference only, not for distribution.)

INITIAL HJWP MAP DATE
MAY 24, 1974

FLOOD HAZARD BOUNDARY MAP REVISIONS
JANUARY 7, 1977

FLOOD INSURANCE RATE MAP EFFECTIVE

[illegible]



 MAP SCALE 1" = 500'

 250 0 500 1000

 FEET METERS



 PANEL 0112E

FIKIM
FLOOD INSURANCE RATE MAP
CARSON CITY

CARSON CITY,
NEVADA
INDEPENDENT CITY
PANEL 112 OF 275

INSURANCE	CONTAINS:	COMMUNITY	NUMBER	PANEL	SUFFIX
		CARSON CITY	300000	012	E

FLOOD

Notice to User: The Map Number shown below should be used for insurance applications for the subject community.

MAP NUMBER
3200010112E

MAP REVISED



INTERNATIONAL
ASSOCIATION OF
FIRE MARSHALS


JANUARY 16, 2009
Federal Emergency Management Agency



Appendix A: Sewage Generation Estimates



241 Ridge Street, Suite 400, Reno, Nevada 89501
(775)746-3500

Project: Plateau Development TM
Subject: Sewer Loading Estimate
Client: Tahoe IV LLC
Job Code: TIV.CCNV01.00
Date: 10/10/2018
By: CAnderson
Checked: RWenziger

PLATEAU TENTATIVE MAP

Goal:

Estimates of proposed sewage generation for Plateau based on the tentative map and adjacent zone changes.

Estimates of existing sewage generation for site based on the existing zoning.

Assumptions:

- Based on Table 5-4 Carson City Sewer Master Plan Update July 2017
- Assume no groundwater infiltration
- Assume no raw acreage offset
- Peaking Factor per Table 5-1 Carson City Sewer Master Plan Update July 2017

Peaking Factor= 1.5 Average Day to Peak Hourly

Average Loading

Single Family Residential=	148 gpd /EDU
Duplex=	148 gpd /EDU
Apartments/Multi-Family (MF)=	123 gpd /EDU
Office=	269 gpd /ac
Commercial=	443 gpd /ac

Average Loading

Hotel/Resort/Casino Hotels/Motels=	62 gpd /room
Industrial=	189 gpd /ac
Institutional=	1127 gpd/ac
School=	158 gpd/ac
Prison=	366 gpd/ac
Hospital=	366 gpd/ac

Existing Zoning

General Industrial	Acres	Lots (EDUs)	Description	Zoning	Average Day		Peak Hourly	
					EDUs	GPD	GPM	GPM
	112.69				0	21,298	15	22
						21,298	15	22

Plateau TM and Zoning Change Estimate

Park/Common Open Space	Acres	Lots or EDUs	Description	Zoning	Average Day		Peak Hourly	
					EDUs	GPD	GPM	GPM
Single Family Residential		23.4	Desert landscaping	SF6	0	0	0.0	0.0
Multi-Family (MF)		67.89		MF	269	39,812	27.6	41.5
General Commercial		18.53		GC	250	30,750	21.4	32.0
		13.61				6,029	4.2	6.3
						76,591	53	80

Calculations

SFR or MF Loading: Ave. GPD=EDU*gpd/EDU
Commercial, Industrial, Office, Institutional, School, Prison, or Hospital Loading: Ave. GPD=Acres*gpd/Acre

Table 5-4 summarizes the average calibrated wastewater unit generation rates categorized by land use and the recommended values for us in future planning and forecasting on wastewater flows. The unit wastewater generation rates for each land use type were primarily determined from using water usage data for the month of February 2016 and assuming a 100% return-to-sewer ratio. Winter water usage is assumed to only include indoor water use, therefore water rates, in theory, are typically considered to equal wastewater flows. These values were further refined with the flow metering data. In addition to the flow from sewer sources, significant groundwater infiltration was observed throughout the City. As discussed in **Section 5.3.2**, it is estimated that approximately 243 gallons/acre of developed land, on average, infiltrated into the sewer system.

Table 5-4 Calibrated and Recommended Future Wastewater Unit Generation Rates

Land Use Category (Land Use Unit)	Unit	Average Flow from Sewer Sources (Qs)	Total Flow, Q _T (Q _s + Q _{cwi})
Single-Family Residential	(gpd/DU)	148	219
Multi-Family Residential	(gpd/DU)	123	140
Office	(gpd/acre)	269	467
Commercial	(gpd/acre)	443	708
Hotel/Resort/Casino Hotels/Motels	(gpd/room)	62	68
Industrial	(gpd/acre)	189	564
Institutional	(gpd/acre)	1,127	1,452
School	(gpd/acre)	158	392
Prison	(gpd/acre)	366	463
Hospital	(gpd/acre)	2,333	2,686

Table 5-1 Temporary and Permanent Flow Metering Summary

Meter Name	Meter Type	Average Daily Flow (mgd)	Average Hourly Peak Flow (mgd)	Sewer Peaking Factor
Meter 01	Temporary	0.740	1.036	1.4
Meter 02	Temporary	2.016	2.744	1.4
Meter 03	Temporary	2.212	2.877	1.3
Meter 04	Temporary	1.169	1.367	1.2
Meter 05	Temporary	0.076	0.112	1.5
North Lift Station Meter	Permanent	4.129	5.671	1.5
Headworks Meter	Permanent	6.557	8.012	1.2

Notes:

- Permanent and temporary meter data was collected during February 16 to March 9, 2017.
- The data excludes metered data collected during February 20- 22 and March 5 due to the influence of wet weather flows. Refer to **Section 5.3.3** for a general discussion on wet weather flow analysis.



Appendix B: FlowMaster Pipe Capacity Calculations

TIVCCNV01 Plateau Development Average Daily Flow Capacity

Project Description

Friction Method	Manning Formula
Solve For	Normal Depth

Input Data

Roughness Coefficient	0.010	
Channel Slope	0.40	%
Diameter	8.00	in
Discharge	27.60	gal/min

Results

Normal Depth	1.35	in
Flow Area	0.04	ft ²
Wetted Perimeter	0.56	ft
Hydraulic Radius	0.83	in
Top Width	0.50	ft
Critical Depth	0.11	ft
Percent Full	16.9	%
Critical Slope	0.00400	ft/ft
Velocity	1.58	ft/s
Velocity Head	0.04	ft
Specific Energy	0.15	ft
Froude Number	1.00	
Maximum Discharge	1.07	ft ³ /s
Discharge Full	0.99	ft ³ /s
Slope Full	0.00002	ft/ft
Flow Type	SubCritical	

GVF Input Data

Downstream Depth	0.00	in
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.00	in
Profile Description		
Profile Headloss	0.00	ft
Average End Depth Over Rise	0.00	%
Normal Depth Over Rise	16.86	%
Downstream Velocity	Infinity	ft/s

TIVCCNV01 Plateau Development Average Daily Flow Capacity

GVF Output Data

Upstream Velocity	Infinity	ft/s
Normal Depth	1.35	in
Critical Depth	0.11	ft
Channel Slope	0.40	%
Critical Slope	0.00400	ft/ft

TIVCCNV01 Plateau Development Peak Hourly Flow Capacity

Project Description

Friction Method	Manning Formula
Solve For	Normal Depth

Input Data

Roughness Coefficient	0.010	
Channel Slope	0.40	%
Diameter	8.00	in
Discharge	41.50	gal/min

Results

Normal Depth	1.65	in
Flow Area	0.05	ft ²
Wetted Perimeter	0.63	ft
Hydraulic Radius	0.99	in
Top Width	0.54	ft
Critical Depth	0.14	ft
Percent Full	20.6	%
Critical Slope	0.00392	ft/ft
Velocity	1.78	ft/s
Velocity Head	0.05	ft
Specific Energy	0.19	ft
Froude Number	1.01	
Maximum Discharge	1.07	ft ³ /s
Discharge Full	0.99	ft ³ /s
Slope Full	0.00003	ft/ft
Flow Type	SuperCritical	

GVF Input Data

Downstream Depth	0.00	in
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.00	in
Profile Description		
Profile Headloss	0.00	ft
Average End Depth Over Rise	0.00	%
Normal Depth Over Rise	20.61	%
Downstream Velocity	Infinity	ft/s

TIVCCNV01 Plateau Development Peak Hourly Flow Capacity

GVF Output Data

Upstream Velocity	Infinity	ft/s
Normal Depth	1.65	in
Critical Depth	0.14	ft
Channel Slope	0.40	%
Critical Slope	0.00392	ft/ft